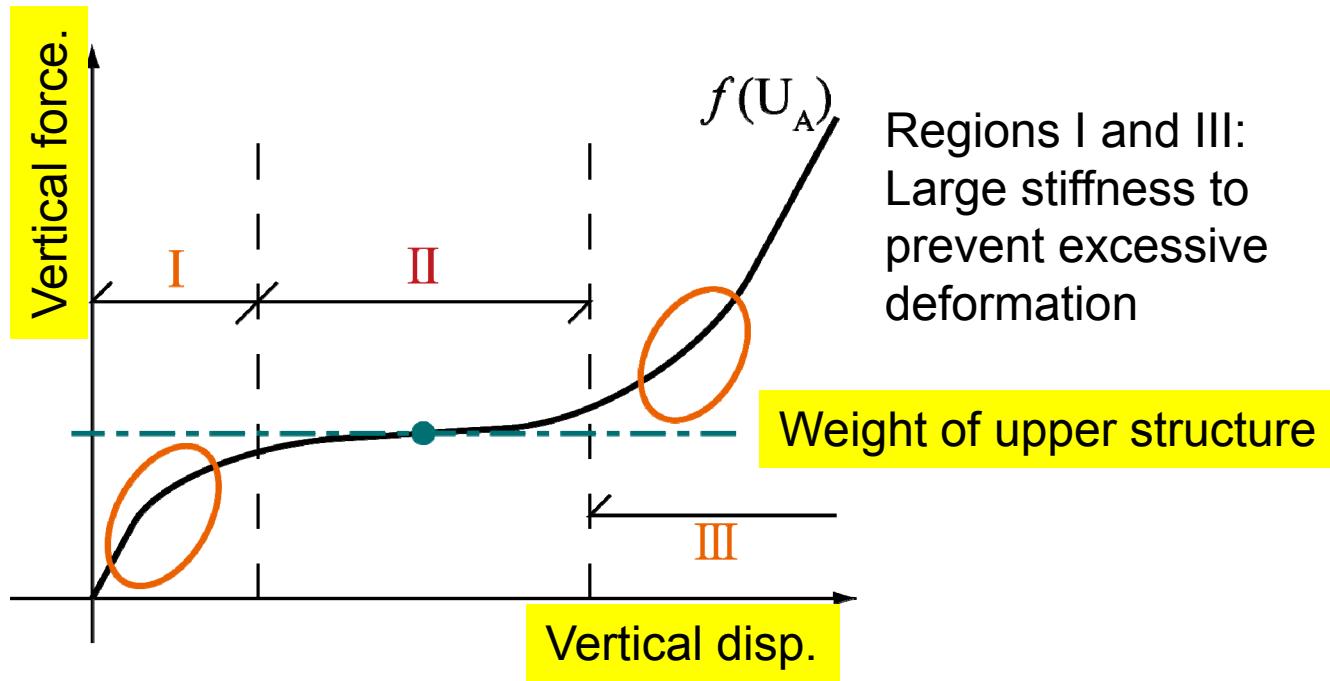


Optimization of flexible structure for vertical base isolation

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Yakuya Kinoshita
(Kyoto Univ., Japan
currently Takenaka Corp.)



Region II:
Support upper structure with small
stiffness

Regions I and III:
Large stiffness to
prevent excessive
deformation

Weight of upper structure

Problem formulation

$$\text{minimize} \quad e(X, A) = \sum_{i=1}^m \left(f(U_A^{(i)}) - \bar{f}_i \right)^2$$

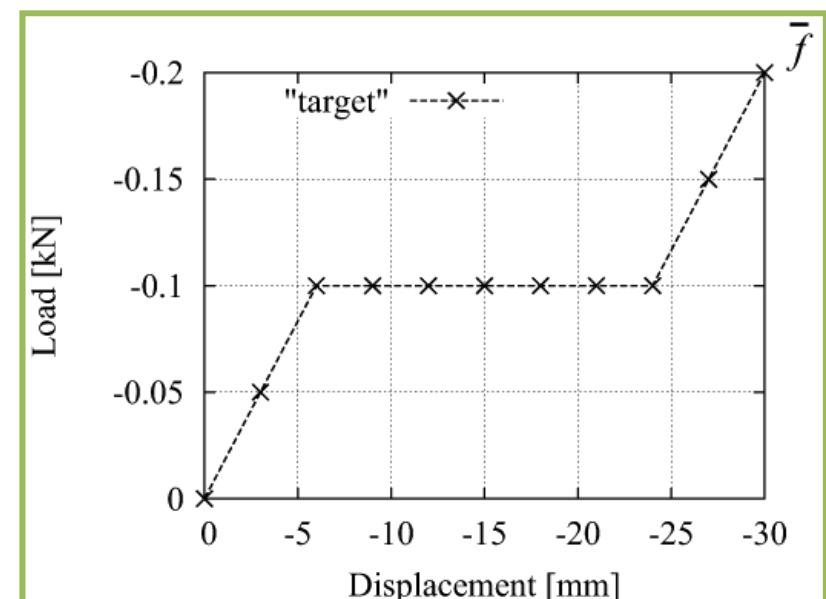
subject to $f(U_A^{(i)}) \leq f(U_A^{(i+1)})$
 $(i = l, \dots, u-1)$

$$V(A, X) \leq V^{\max}$$

$$A^L \leq A \leq A^U$$

$$X^L < X < X^U$$

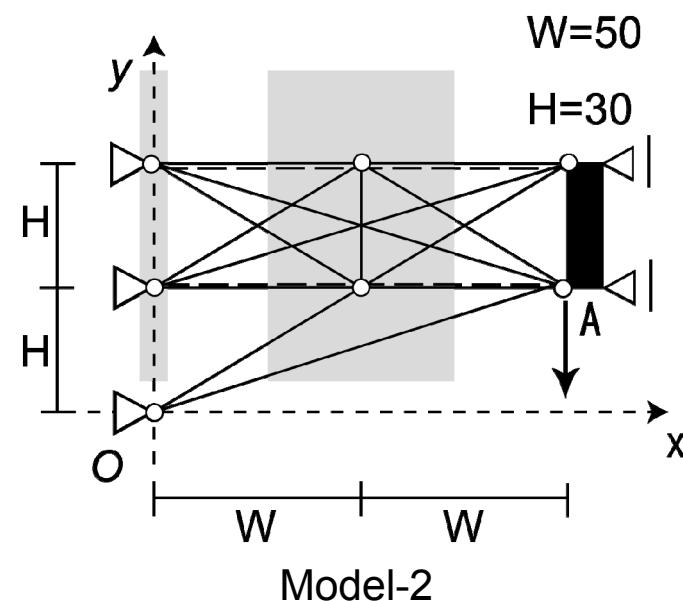
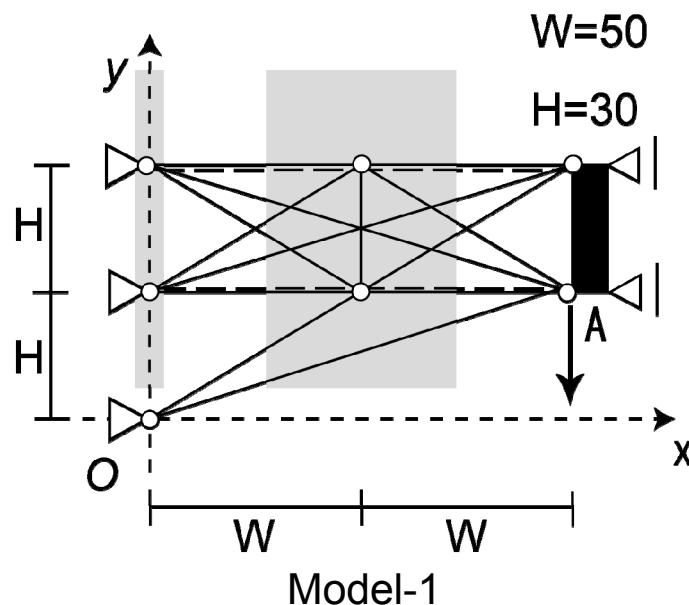
Minimize error from specified path



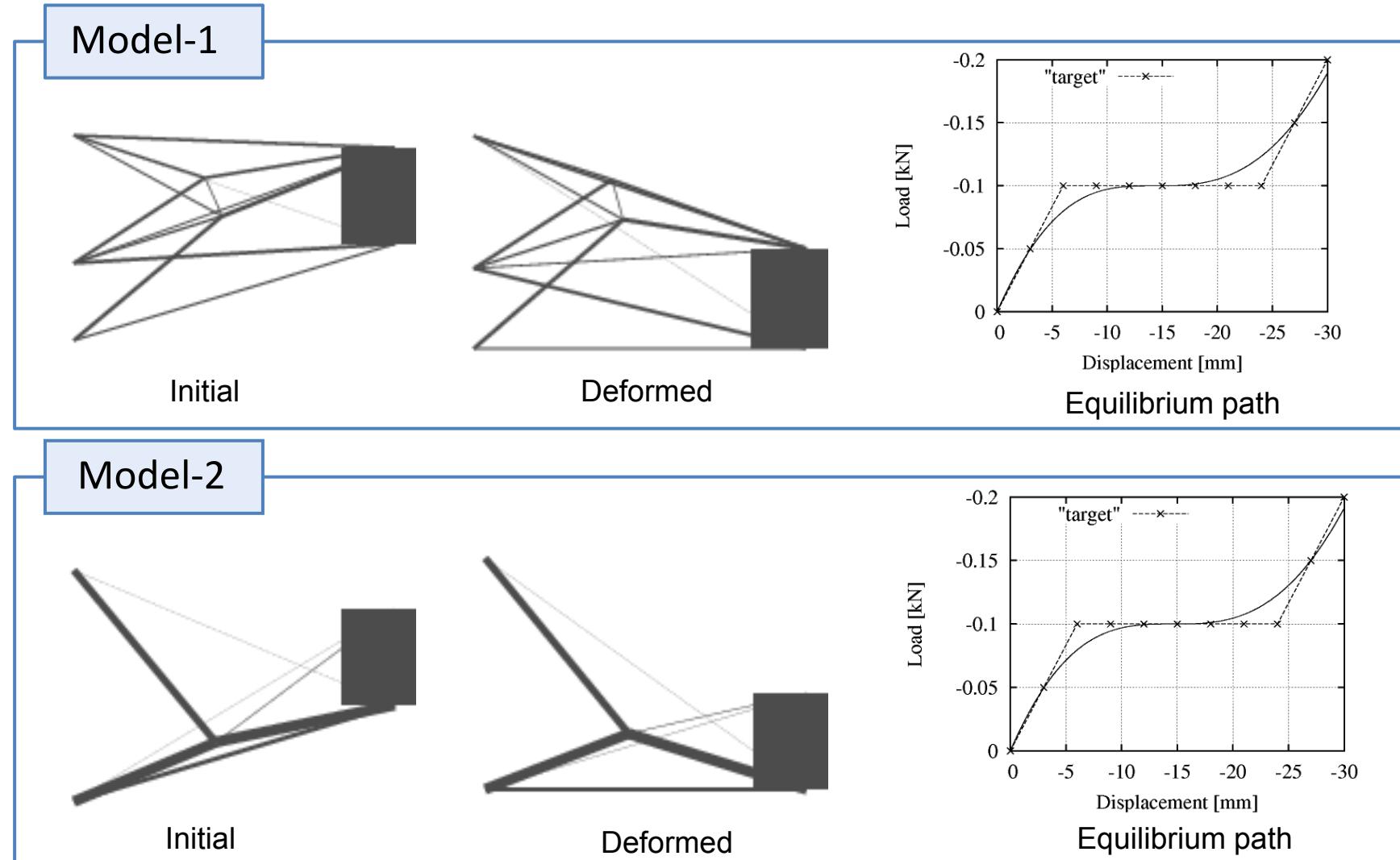
Specified path

Truss model

- Models 1 and 2
 - Ground structure with truss elements
 - Add rigid element at right supports to represent upper structure
 - Optimize from several random initial solutions



Optimization results

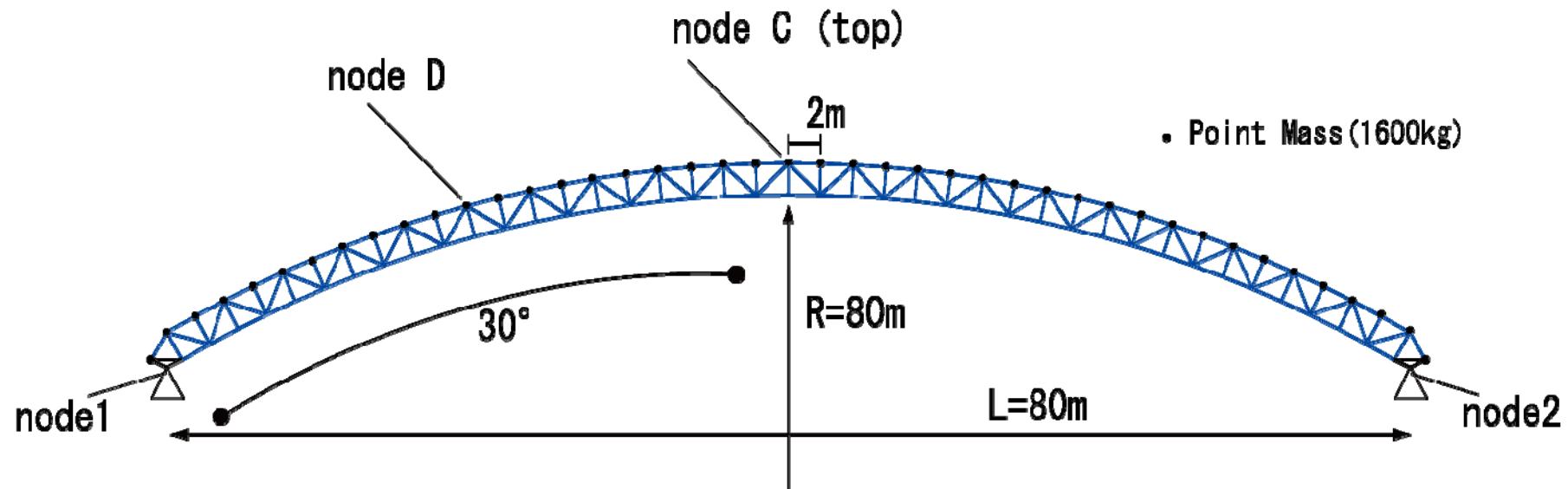


Upper structure

Arch model



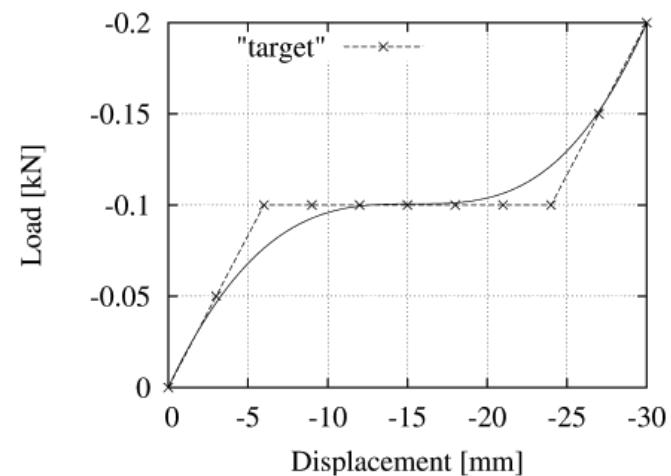
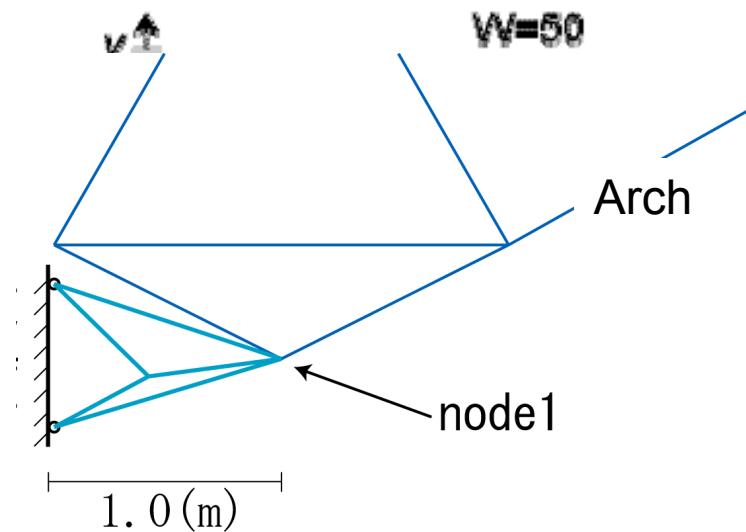
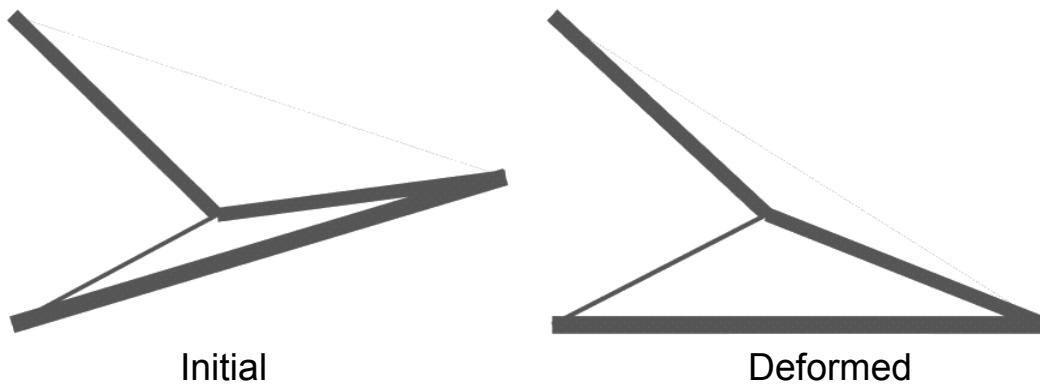
$$T_1 = 1.21, T_2 = 2.27, T_3 = 3.88, T_4 = 4.52$$



Base isolation model

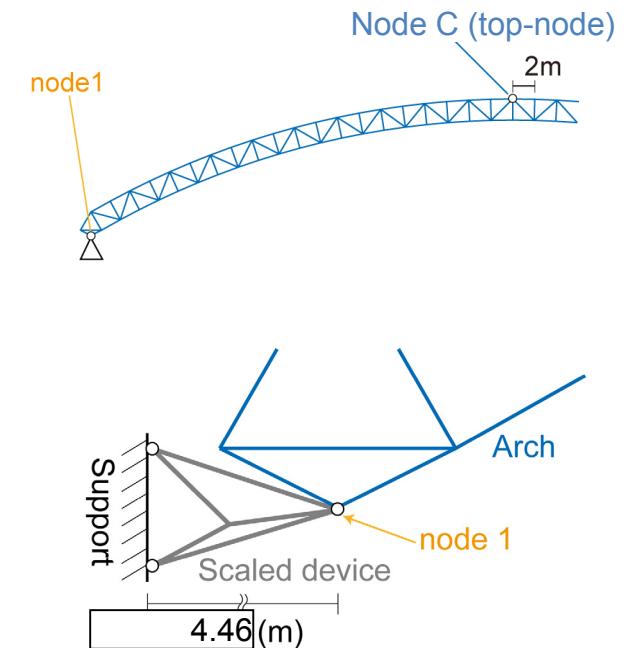
- Specification

- Width 1.0 (m)
- Young's modulus 2.0×10^4 (N/mm²)
- Cross-sectional area:
multiply 3.21×10^2 so that the self-weight is supported at the center of Region II



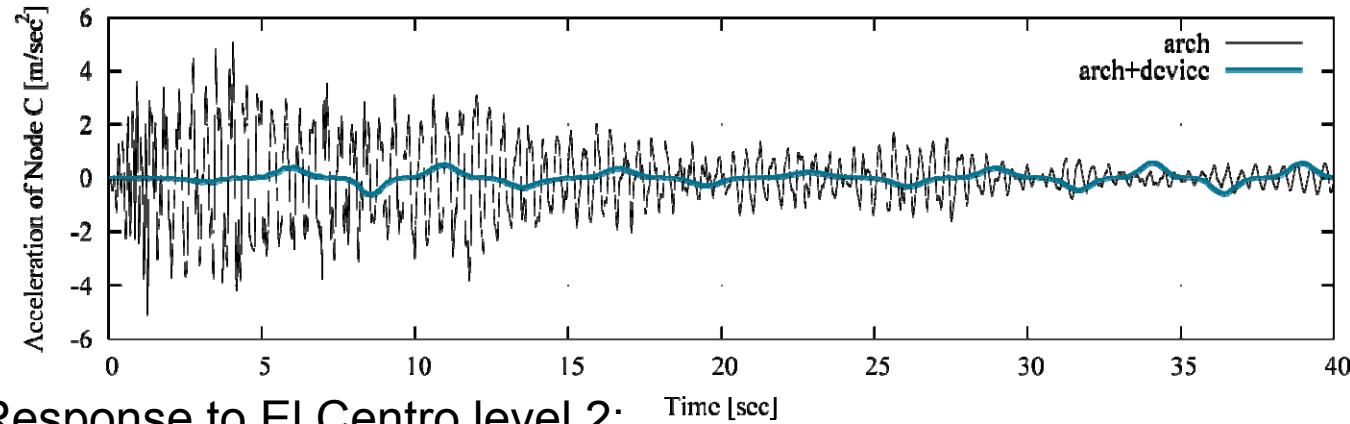
Response against recorded ground motions

- (i) El Centro UD (1940)
 - (ii) Hachinohe UD(1968)
 - (iii) Taft UD (1952)
 - (iv) Takatori UD (1995) ... Original level
-]
- Levels 2 and 3



- Bar-spring model
 - Max. deformation of spring: 0.402 m
 - Scale of isolation device: $(2 \times 0.402)/0.18 = 4.46$
 - Size of device: width = 4.46 m, height = 2.68 m

Response against recorded ground motions



Response to El Centro level 2:
Blue: isolated, Black: pin-support

地震波	Acc_g^{\max}	Arch		Arch + Spring		応答比	Arch + Device		Response ratio		
		Acc_D^{\max}	Acc_C^{\max}	Δ^{\max}	Acc_D^{\max}	Acc_C^{\max}	Δ^{\max}	Acc_D^{\max}	Acc_C^{\max}		
(i) ₂	3.084	6.004	5.109	0.236	0.580	0.594	0.116	0.290	0.613	0.627	0.123
(ii) ₂	1.627	12.557	6.223	0.135	0.336	0.347	0.056	0.394	1.732	1.803	0.290
(iii) ₂	2.906	12.186	9.524	0.402	0.989	1.010	0.106	0.327	1.039	1.132	0.119
(i) ₃	4.626	8.989	7.651	0.285	0.708	0.706	0.092	0.489	2.977	3.139	0.410
(ii) ₃	2.440	18.860	9.405	0.203	0.505	0.521	0.055	0.408	1.674	1.748	0.186
(iii) ₃	4.359	18.222	14.213	0.473	1.217	1.234	0.087	0.387	1.417	1.464	0.103
(iv)	2.793	9.403	7.123	0.102	0.263	0.269	0.038	0.133	0.108	0.110	0.015